

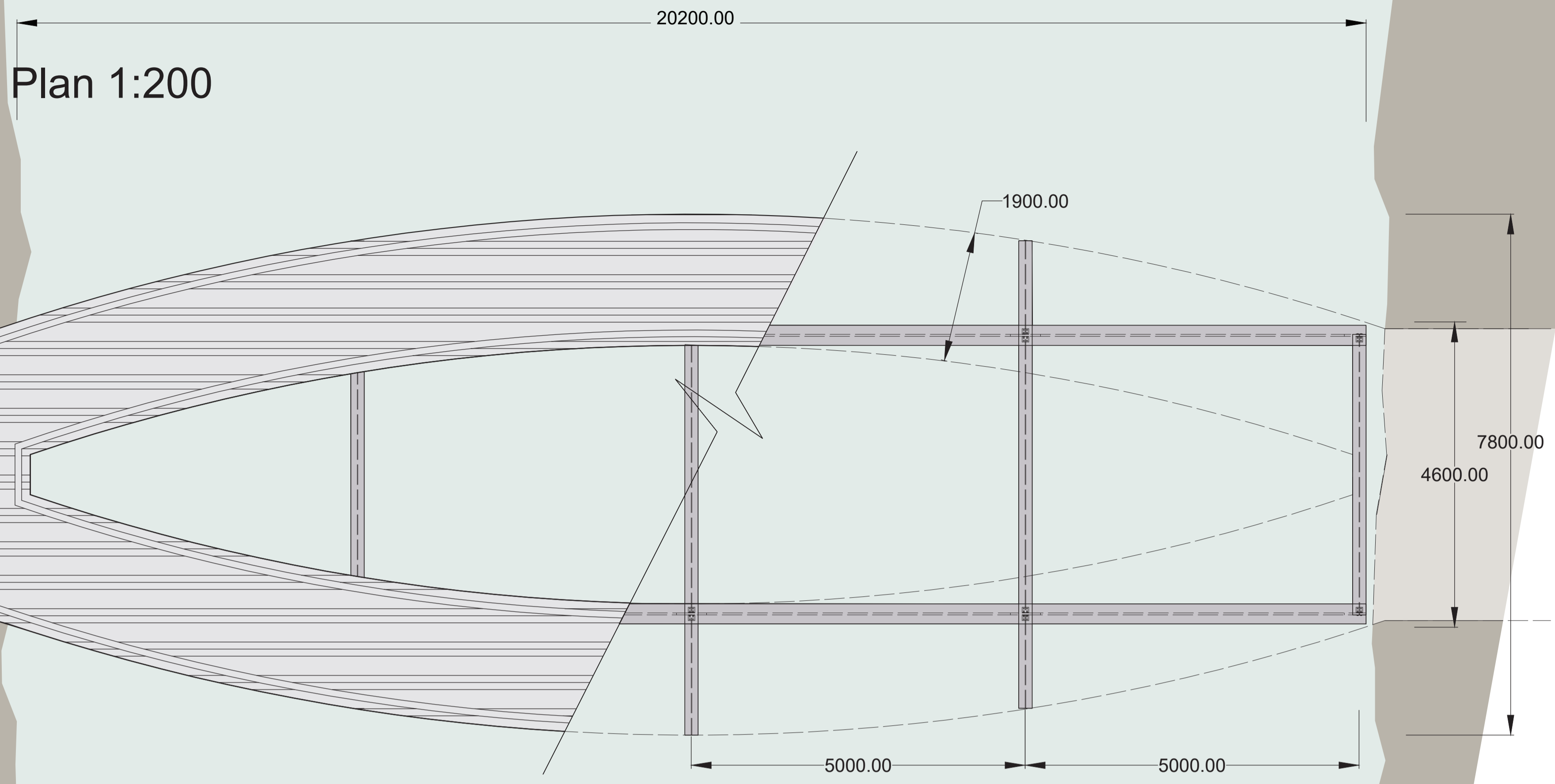


Bachelor's Degree in Architecture

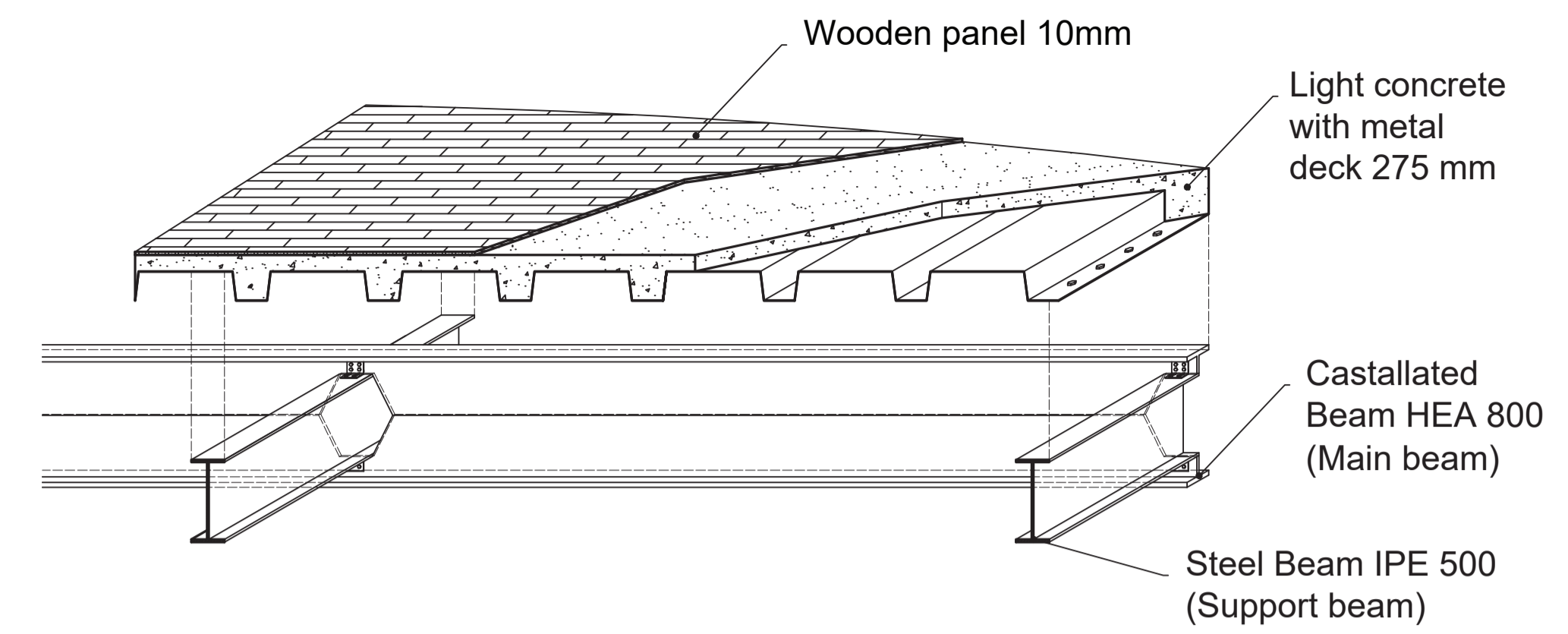
Fundamentals of structural analysis/ *Fabrizio Barpi*

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Bridge Plan 1:200



Only the metal decking will be shaped with curvature.

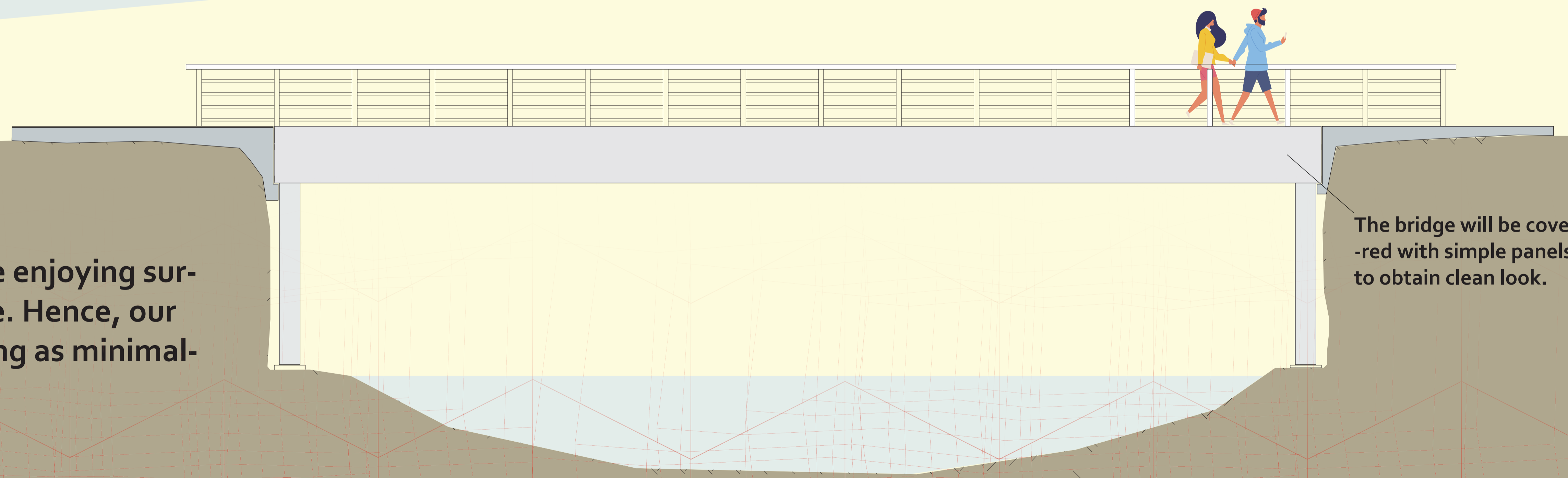


Castallated beam will allow supporting beams to pass the main beam, making it possible to shape the deck.

Light concrete with metal deck will be installed to beam structure. It will be layered with wooden panel as the top layer.

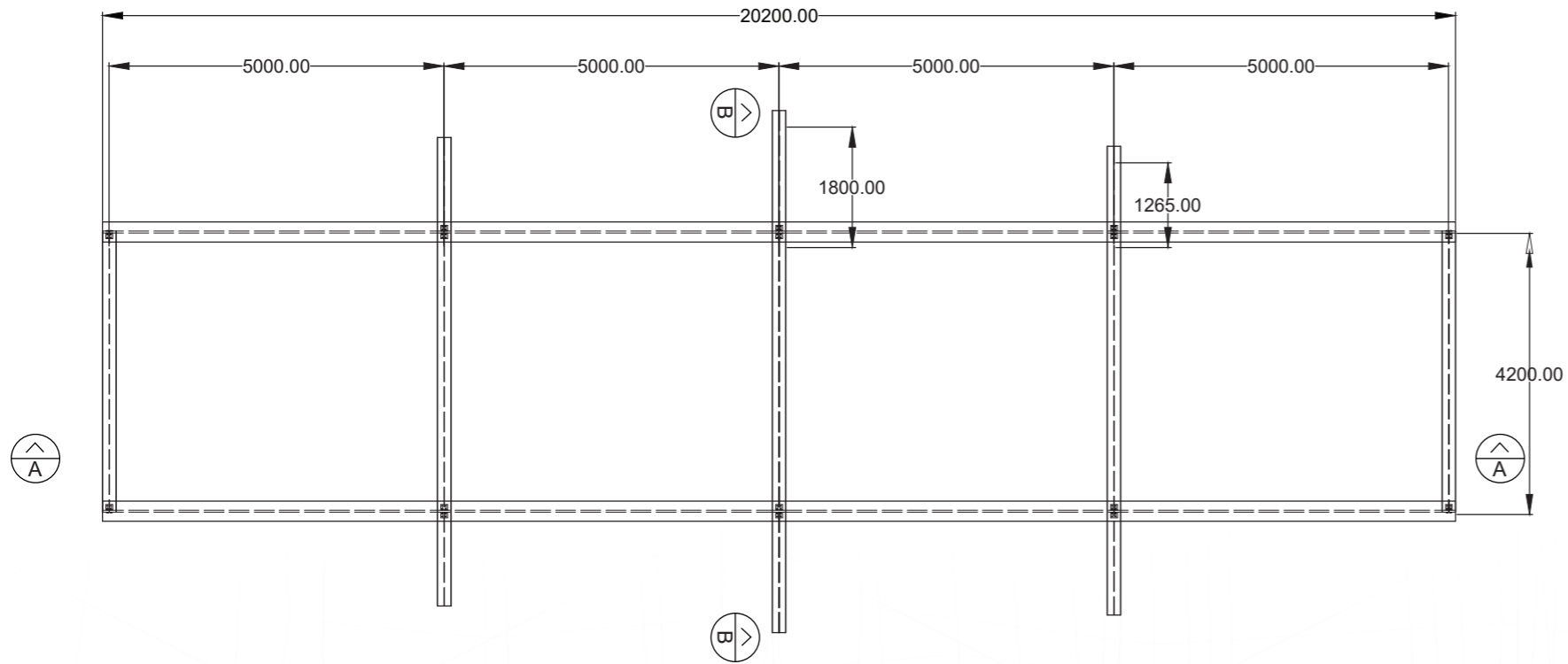
Elevation 1:200

The main goal of the bridge will be enjoying surrounding without any interference. Hence, our design goal was to make everything as minimalist as possible.

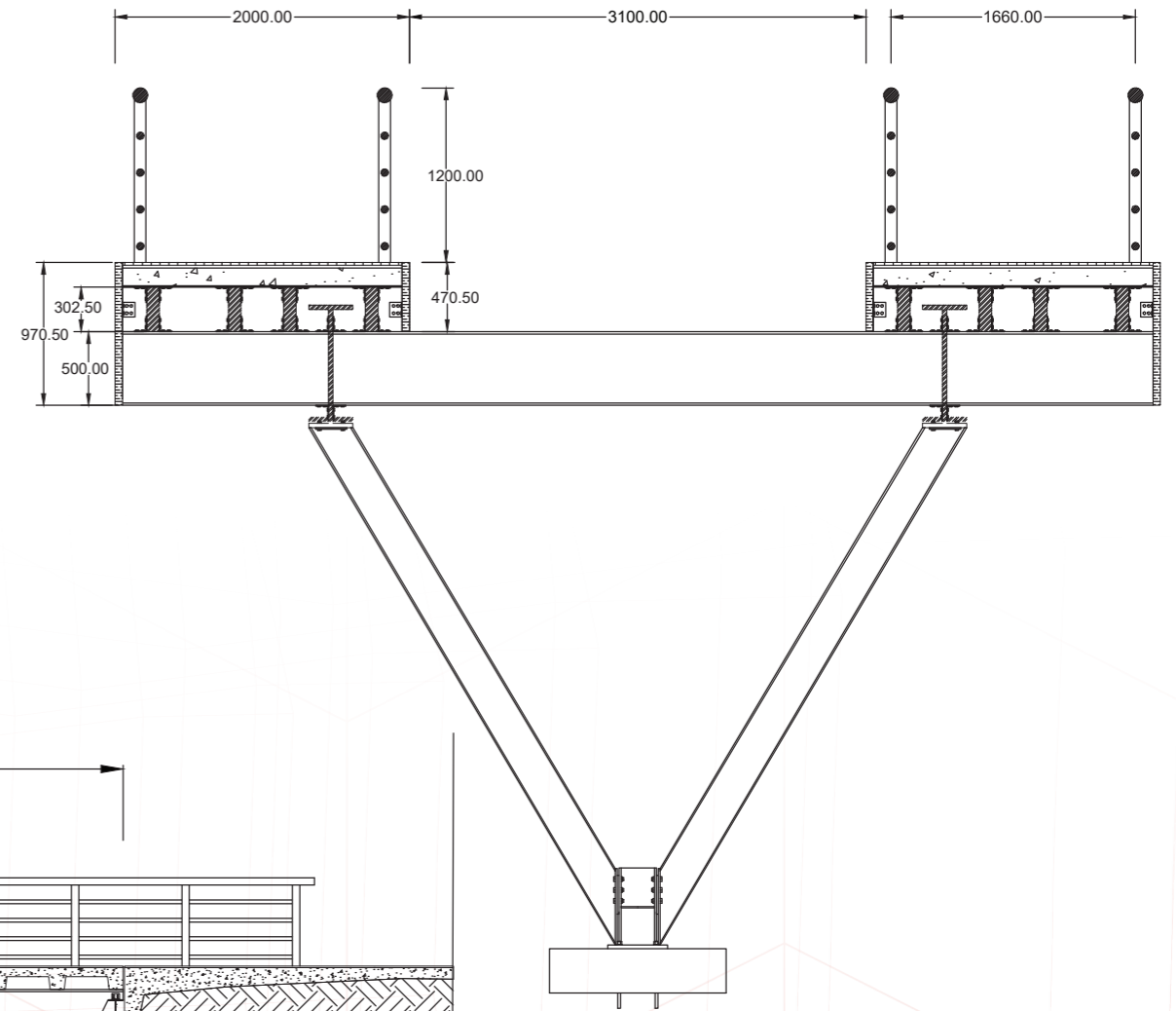


The bridge will be covered with simple panels to obtain clean look.

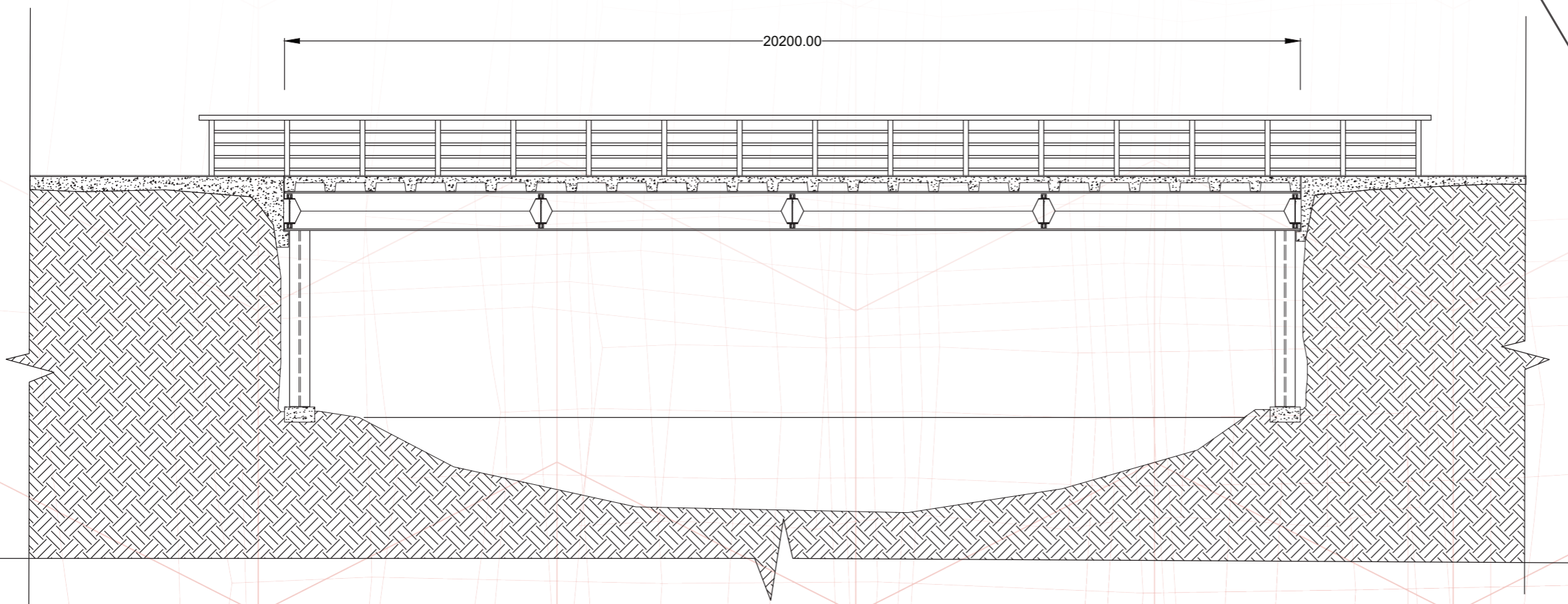
Structural plan of the beam 1:100



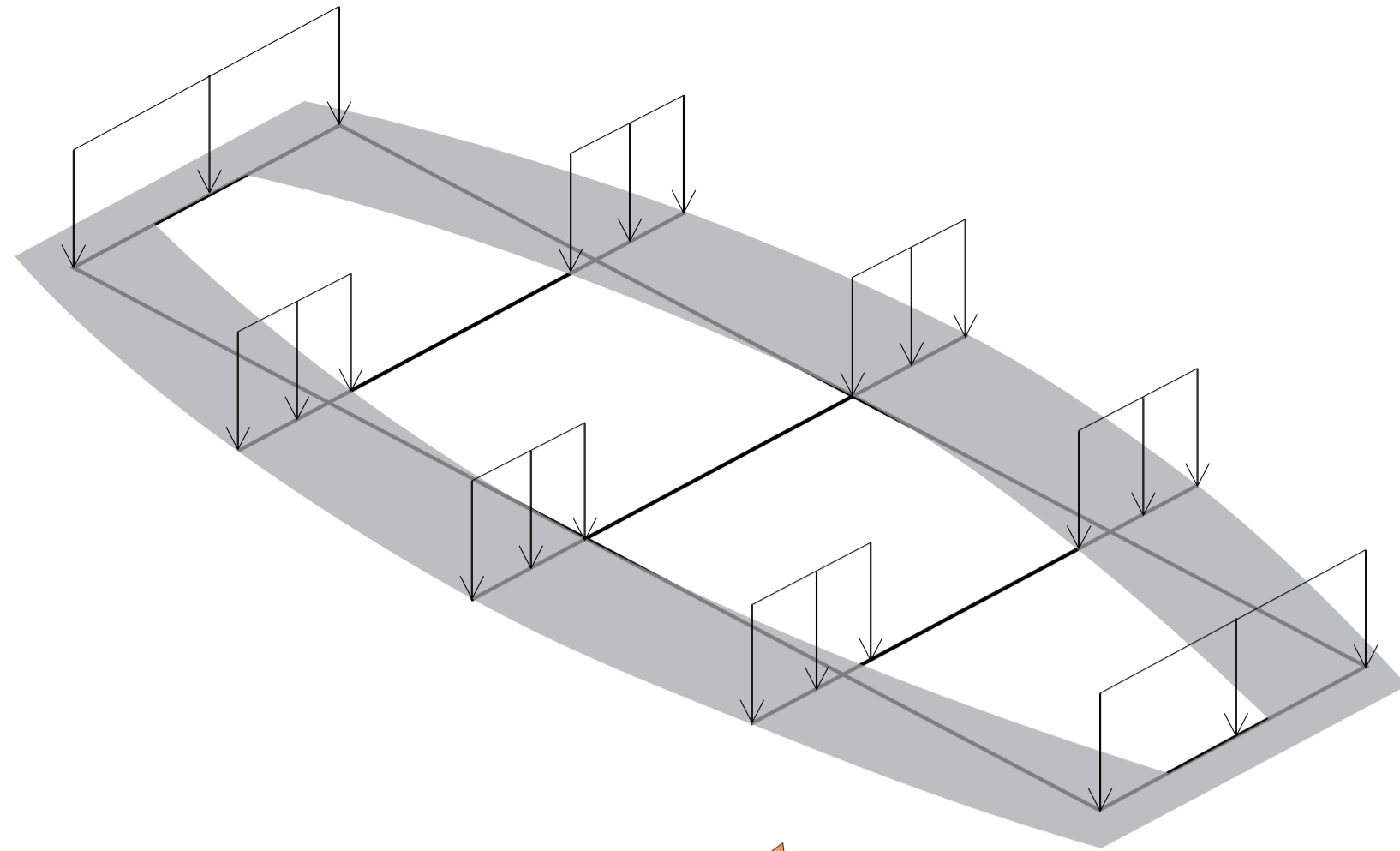
Section B-B 1:50



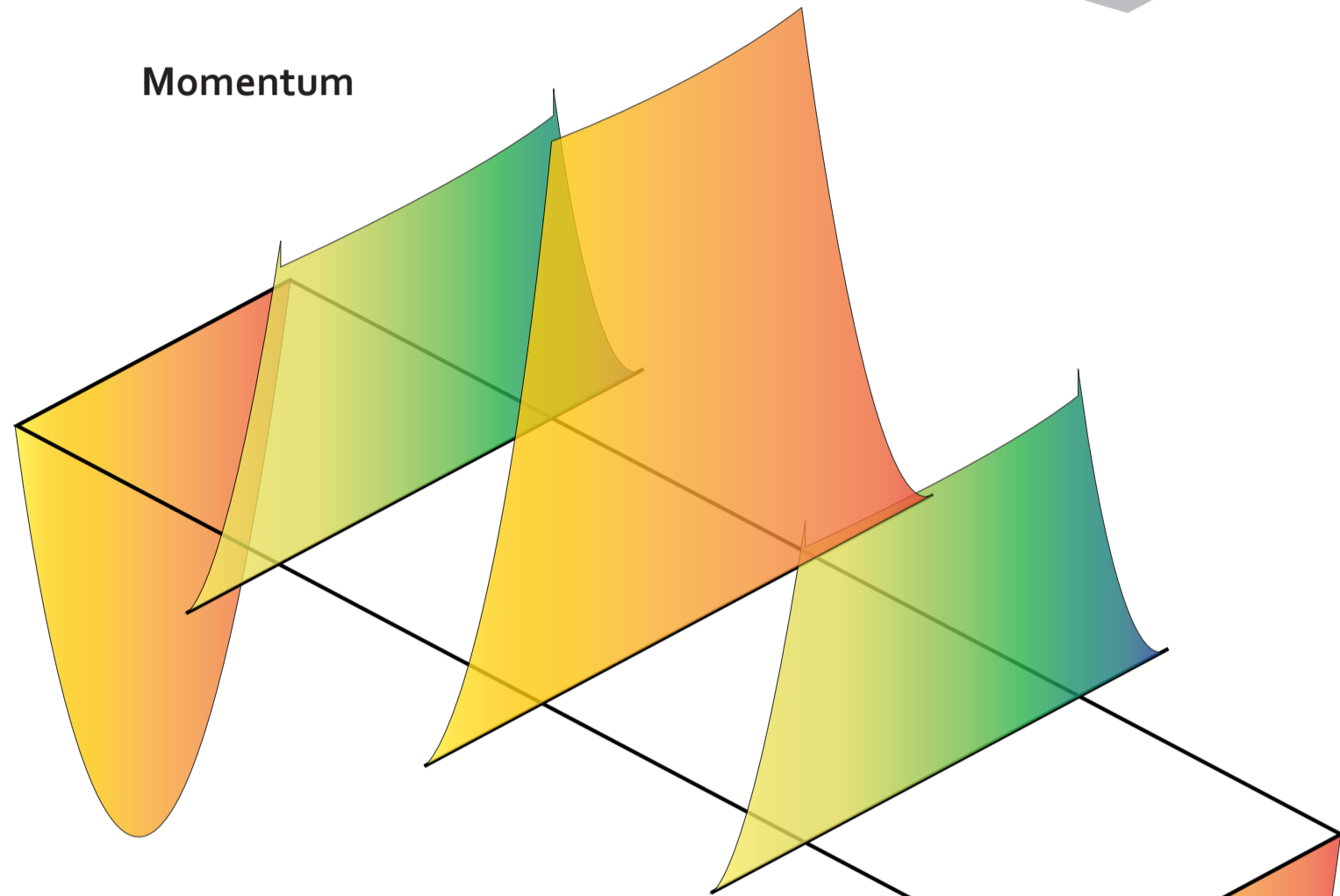
Section A-A 1:100



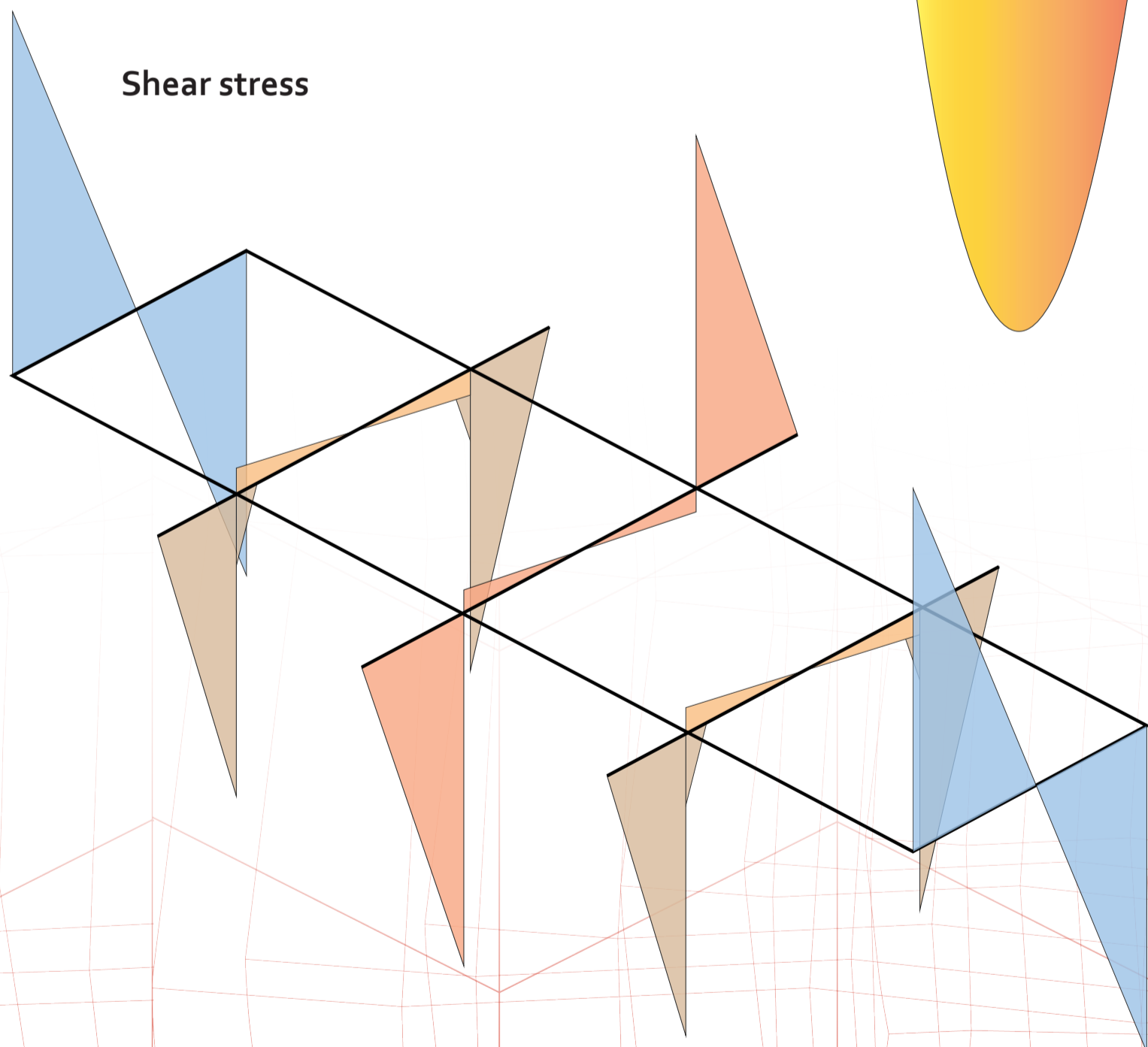
Vertical Forces



Momentum



Shear stress



CALCULATION OF THE BEAMS

Support beam 1

$\Sigma F_x = 0$: HB = 6050 (kN) $\Sigma M_A = 0$:
 The sum of the moments about the roller support at the point A:
 $P1 \cdot 1.8 + q1 \cdot 1.5 \cdot (1.8 - 1.5/2) + P2 \cdot 0.3 + RB \cdot 4.2 - P3 \cdot 4.5 - q2 \cdot 1.5 \cdot (4.5 + 1.5/2) - P4 \cdot 6 = 0$
 $\Sigma M_B = 0$: The sum of the moments about the pin support at the point B:
 $P1 \cdot 6 + q1 \cdot 1.5 \cdot (6 - 1.5/2) + P2 \cdot 4.5 - RA \cdot 4.2 - P3 \cdot 0.3 - q2 \cdot 1.5 \cdot (0.3 + 1.5/2) - P4 \cdot 1.8 = 0$
 2. Calculate reaction of pin support at the point B:
 $RB = (-P1 \cdot 1.8 - q1 \cdot 1.5 \cdot (1.8 - 1.5/2) - P2 \cdot 0.3 + P3 \cdot 4.5 + q2 \cdot 1.5 \cdot (4.5 + 1.5/2) + P4 \cdot 6) / 4.2 = (-189 \cdot 1.8 - 10.33 \cdot 1.5 \cdot (1.8 - 1.5/2) - 189 \cdot 0.3 + 189 \cdot 4.5 + 10.33 \cdot 1.5 \cdot (4.5 + 1.5/2) + 189 \cdot 6) / 4.2 = 393.50$ (kN)
 3. Calculate reaction of roller support at the point A:
 $RA = (P1 \cdot 6 + q1 \cdot 1.5 \cdot (6 - 1.5/2) + P2 \cdot 4.5 - P3 \cdot 0.3 - q2 \cdot 1.5 \cdot (0.3 + 1.5/2) - P4 \cdot 1.8) / 4.2 = (189 \cdot 6 + 10.33 \cdot 1.5 \cdot (6 - 1.5/2) + 189 \cdot 4.5 - 189 \cdot 0.3 - 10.33 \cdot 1.5 \cdot (0.3 + 1.5/2) - 189 \cdot 1.8) / 4.2 = 393.50$ (kN)
 4. Solve this system of equations:
 HB = 6050 (kN)
 5. The sum of the forces about the Oy axis is zero:
 $\Sigma F_y = 0$: $-P1 - q1 \cdot 1.5 - P2 + RA + RB - P3 - q2 \cdot 1.5 - P4 = -189 - 10.33 \cdot 1.5 - 189 + 393.50 \cdot 1 + 393.50 \cdot 1 - 189 - 10.33 \cdot 1.5 - 189 = 0$

Support beam 2

$\Sigma F_x = 0$: HB = 6050 (kN) $\Sigma M_A = 0$:
 1. The sum of the moments about the roller support at the point A:
 $P1 \cdot 1.4 + q1 \cdot 1.5 \cdot (1.4 - 1.5/2) - P2 \cdot 0.1 - P3 \cdot 4.1 - q2 \cdot 1.5 \cdot (4.1 + 1.5/2) + RB \cdot 4.2 - P4 \cdot 5.6 = 0$
 $\Sigma M_B = 0$: The sum of the moments about the pin support at the point B:
 $P1 \cdot 5.6 + q1 \cdot 1.5 \cdot (5.6 - 1.5/2) - RA \cdot 4.2 + P2 \cdot 4.1 + P3 \cdot 0.1 - q2 \cdot 1.5 \cdot (-0.1 + 1.5/2) - P4 \cdot 1.4 = 0$
 2. Calculate reaction of pin support at the point B:
 $RB = (-P1 \cdot 1.4 - q1 \cdot 1.5 \cdot (1.4 - 1.5/2) + P2 \cdot 0.1 + P3 \cdot 4.1 + q2 \cdot 1.5 \cdot (4.1 + 1.5/2) + P4 \cdot 5.6) / 4.2 = (-189 \cdot 1.4 - 10.33 \cdot 1.5 \cdot (1.4 - 1.5/2) + 189 \cdot 0.1 + 189 \cdot 4.1 + 10.33 \cdot 1.5 \cdot (4.1 + 1.5/2) + 189 \cdot 5.6) / 4.2 = 393.50$ (kN)
 3. Calculate reaction of roller support at the point A:
 $RA = (P1 \cdot 5.6 + q1 \cdot 1.5 \cdot (5.6 - 1.5/2) + P2 \cdot 4.1 + P3 \cdot 0.1 - q2 \cdot 1.5 \cdot (-0.1 + 1.5/2) - P4 \cdot 1.4) / 4.2 = (189 \cdot 5.6 + 10.33 \cdot 1.5 \cdot (5.6 - 1.5/2) + 189 \cdot 4.1 + 189 \cdot 0.1 - 10.33 \cdot 1.5 \cdot (-0.1 + 1.5/2) - 189 \cdot 1.4) / 4.2 = 393.50$ (kN)
 4. Solve this system of equations:
 HB = 6050 (kN)
 5. The sum of the forces about the Oy axis is zero:
 $\Sigma F_y = 0$: $-P1 - q1 \cdot 1.5 + RA - P2 - P3 - q2 \cdot 1.5 + RB - P4 = -189 - 10.33 \cdot 1.5 + 393.50 \cdot 1 - 189 - 189 - 10.33 \cdot 1.5 + 393.50 \cdot 1 - 189 = 0$

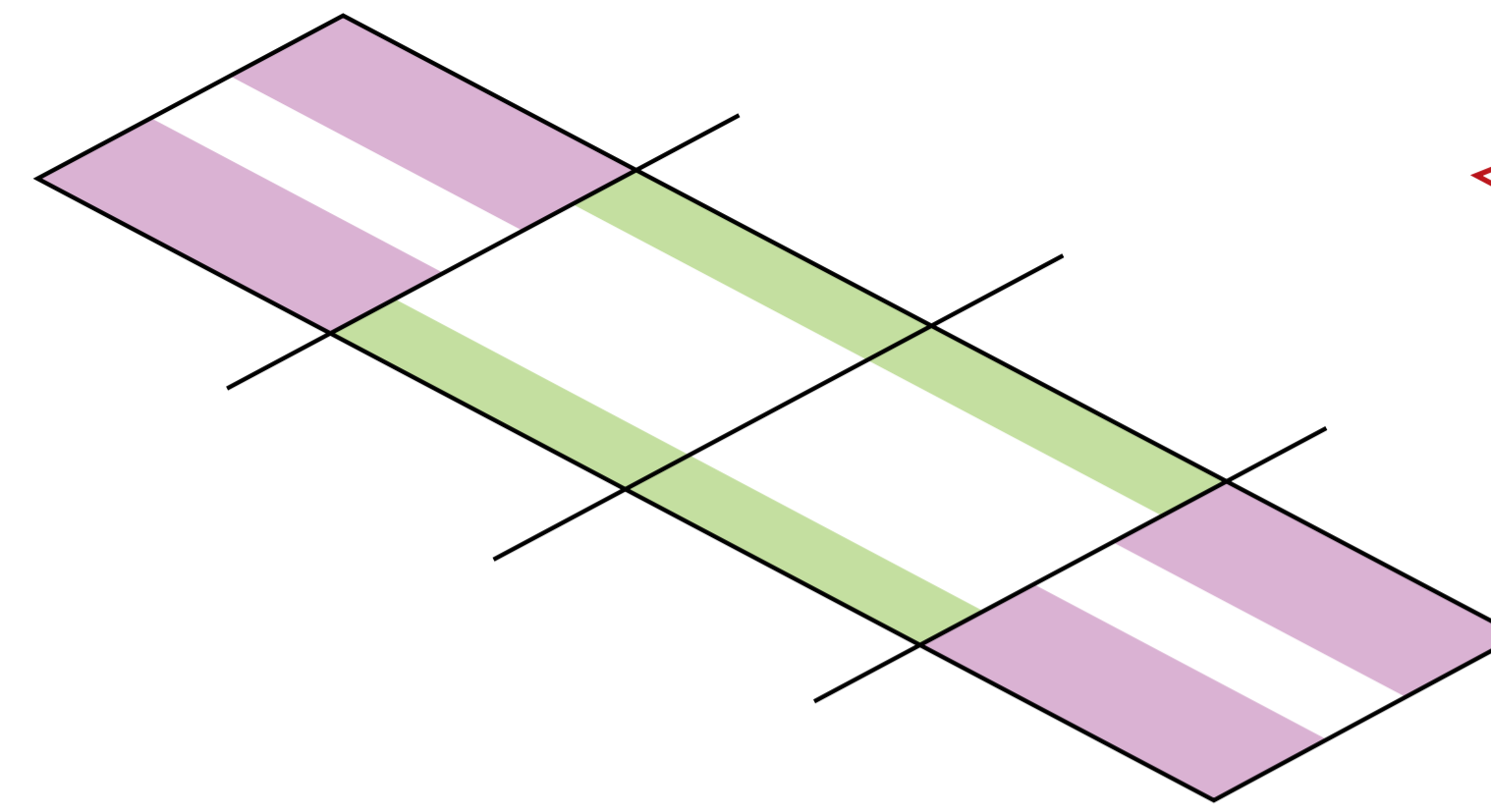
Support beam 3

$\Sigma F_x = 0$: HB = 0 $\Sigma M_A = 0$:
 1. The sum of the moments about the pin support at the point A:
 $-q1 \cdot 4.2 \cdot (4.2/2) + RA \cdot 4.2 - P2 \cdot 4.2 = 0$
 $\Sigma M_B = 0$: The sum of the moments about the roller support at the point B:
 $-RB \cdot 4.2 + P1 \cdot 4.2 + q1 \cdot 4.2 \cdot (4.2 - 4.2/2) = 0$
 2. Calculate reaction of roller support at the point B:
 $RB = (q1 \cdot 4.2 \cdot (4.2/2) + P2 \cdot 4.2) / 4.2 = (10.33 \cdot 4.2 \cdot (4.2/2) + 189 \cdot 4.2) / 4.2 = 210.69$ (kN)
 3. Calculate reaction of pin support at the point A:
 $RA = (P1 \cdot 4.2 + q1 \cdot 4.2 \cdot (4.2 - 4.2/2)) / 4.2 = (189 \cdot 4.2 + 10.33 \cdot 4.2 \cdot (4.2 - 4.2/2)) / 4.2 = 210.69$ (kN)
 4. Solve this system of equations:
 HB = 0 (kN)
 5. The sum of the forces about the Oy axis is zero:
 $\Sigma F_y = 0$: $RB - P1 - q1 \cdot 4.2 + RA - P2 = 210.69 \cdot 1 - 189 - 10.33 \cdot 4.2 + 210.69 \cdot 1 - 189 = 0$

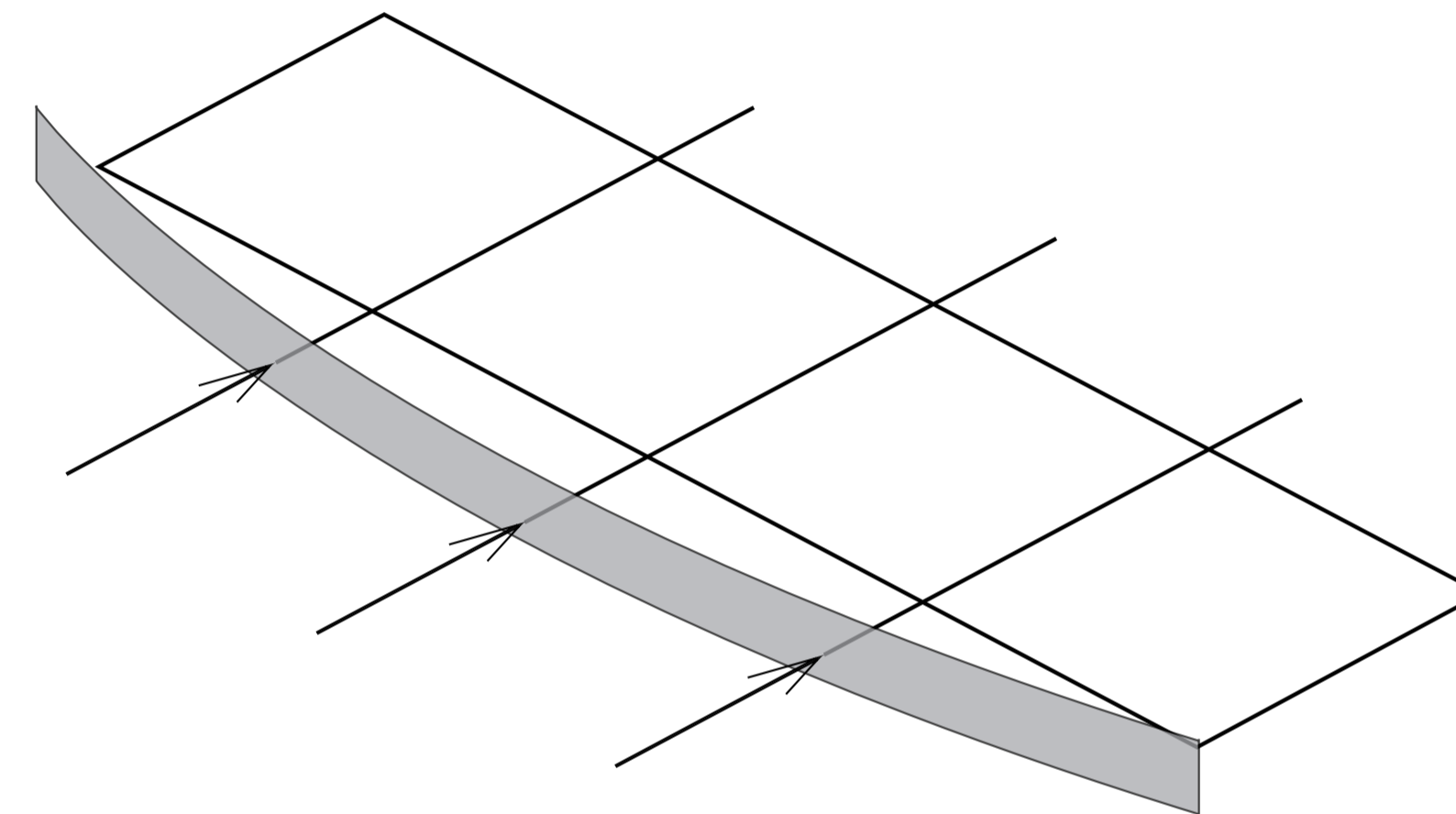
Main beam

$\Sigma F_x = 0$: HB = 18150 (kN) $\Sigma M_A = 0$:
 1. The sum of the moments about the pin support at the point A:
 $-P2 \cdot 5 - P3 \cdot 10 - P4 \cdot 15 + RA \cdot 20 - P5 \cdot 20 = 0$
 $\Sigma M_B = 0$:
 The sum of the moments about the roller support at the point B:
 $-RB \cdot 20 + P1 \cdot 20 + P2 \cdot 15 + P3 \cdot 10 + P4 \cdot 5 = 0$
 2. Calculate reaction of roller support at the point B:
 $RB = (P2 \cdot 5 + P3 \cdot 10 + P4 \cdot 15 + P5 \cdot 20) / 20 = (1470 \cdot 5 + 1805 \cdot 10 + 1470 \cdot 15 + 78 \cdot 20) / 20 = 2450.50$ (kN)
 3. Calculate reaction of pin support at the point A:
 $RA = (P1 \cdot 20 + P2 \cdot 15 + P3 \cdot 10 + P4 \cdot 5) / 20 = (78 \cdot 20 + 1470 \cdot 15 + 1805 \cdot 10 + 1470 \cdot 5) / 20 = 2450.50$ (kN)
 4. Solve this system of equations:
 HB = 18150 (kN)
 5. The sum of the forces about the Oy axis is zero:
 $\Sigma F_y = 0$: $RB - P1 - P2 - P3 - P4 + RA - P5 = 2450.50 \cdot 1 - 78 - 1470 - 1805 - 1470 + 2450.50 \cdot 1 - 78 = 0$

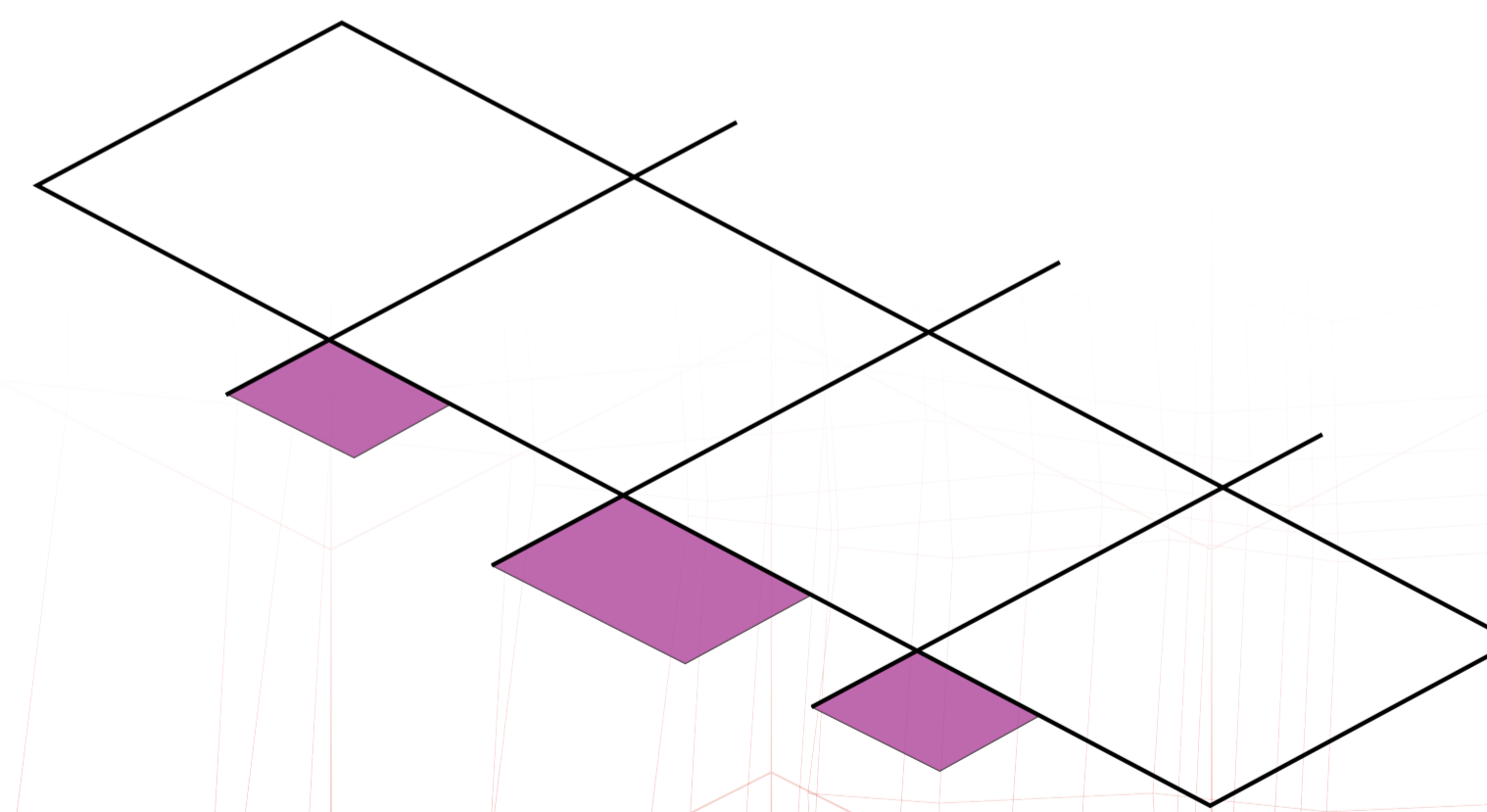
Torsion of the vertical forces



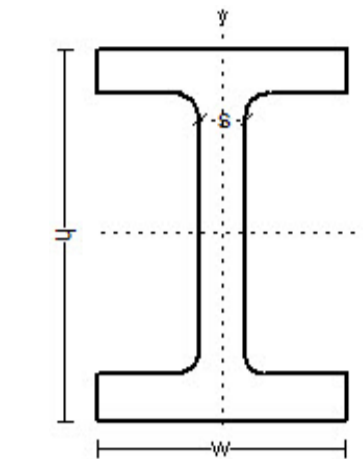
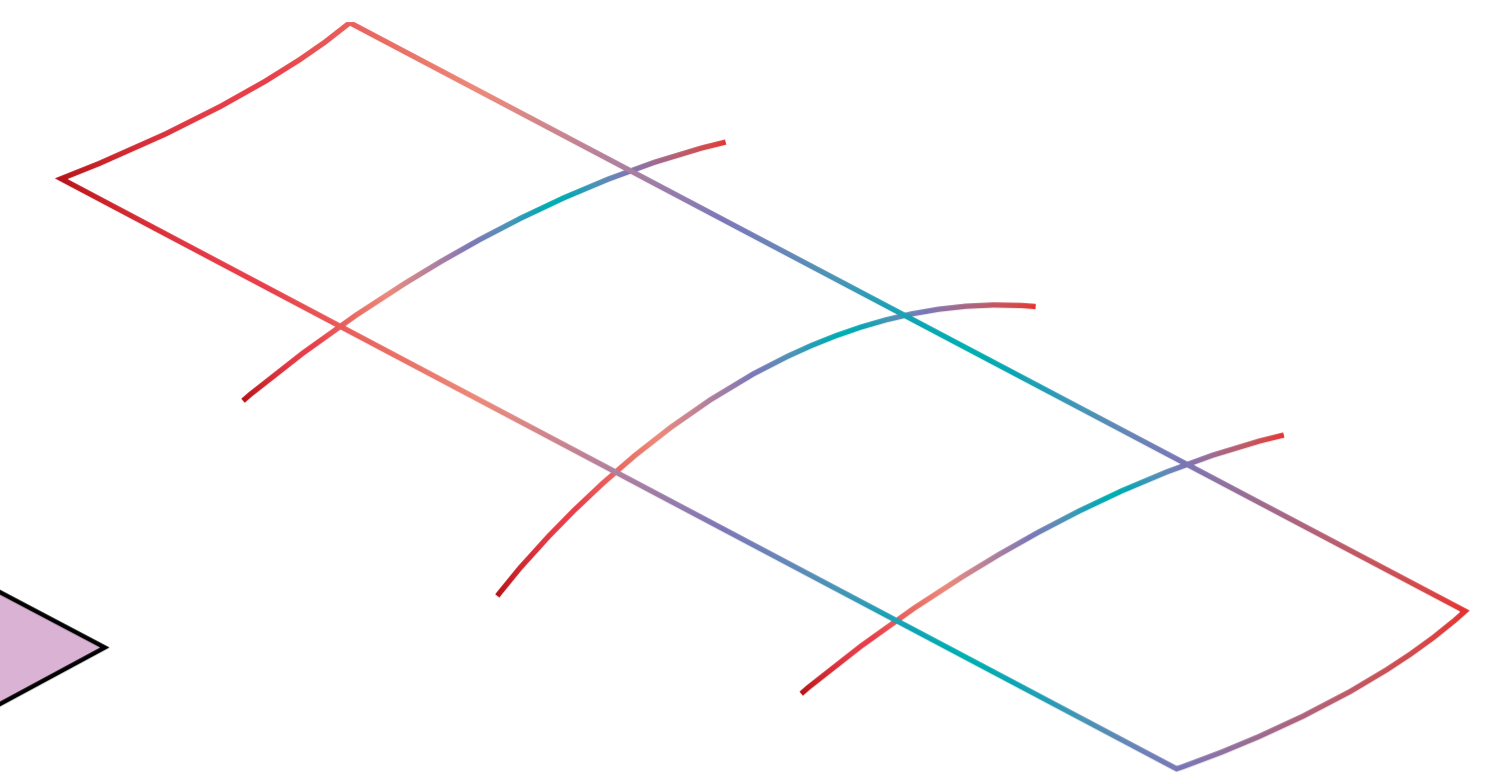
Horizontal forces



Horizontal



Deformation



h = 500mm,
 w = 200mm,
 s = 10.2mm,
 t = 16mm,
 IPE 500

For the slenderness value, we have used IPE 500 for the support beam 1

Critical load
 $P = (\pi^2 \cdot E \cdot I) / (K \cdot L)^2$

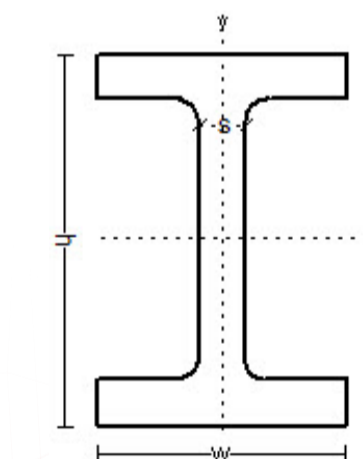
We have chosen support beam 1 for its length
 L = 7.8m K = 2.1 E = 210,000 MPa
 I = 48600 P = 37504

Slenderness
 $\lambda = L / K_{min}$

$K_{min} = \sqrt{I/A} = \sqrt{48600 / 62.6} = 20.46$
 $\lambda = 780 / 20.46 = 381.0706$

Max deformation
 $\sigma = M / I \cdot y$
 M = 65000 Nm² Y = 0.25 m

$\sigma = 65000 \cdot 48600 / 25 = 208.33$
 $\sigma_y \cdot 0.8 = 390 \cdot 0.8 = 312$ $\sigma < \sigma_y$



h = 800mm,
 w = 190mm,
 s = 14.6mm,
 t = 9.4mm,
 HEA 800

For the slenderness value, we have used HEA 800 for the Main beam

Slenderness
 $\lambda = L / K_{min}$

$K_{min} = \sqrt{I/A} = \sqrt{303442 / 286} = 32.57$
 $\lambda = 2000 / 32.57 = 307.0048$

Max deformation
 $\sigma = M / I \cdot y$
 M = 75922 Nm² Y = 0.4 m

$\sigma = 75922 \cdot 303442 / 40 = 151.844$
 $\sigma_y \cdot 0.8 = 390 \cdot 0.8 = 312$ $\sigma < \sigma_y$